

Novotechprom TR150

Position: positive

Trapezoidal corrugated profile type:

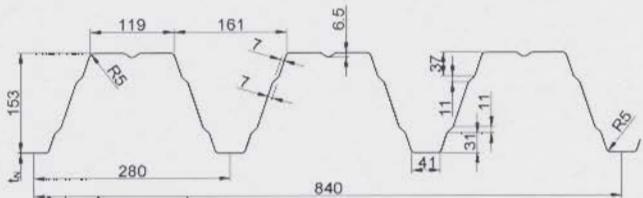
TR 150

Values of crosssection and resistance according DIN 18807 and
the "Anpassungsrichtlinie Stahlbau"

Position-positive

Measurements

in [mm]



Yield strength of steel core $f_y, k = 0.320 \text{ N/mm}^2$

Values of cross-section

thickness of plate t_N [mm]	dead load g [kN/m ²]	bending ¹⁾		normal force stress					limit of span widths ³⁾		
		I_{ef}^+ [cm ⁴ /m]	I_{ef}^- [cm ⁴ /m]	not reduced cross-section			contributive cross-section ²⁾		L_{gr} [m]	single field beam	multiple field beam
				A_g [cm ² /m]	i_g [cm]	z_g [cm]	A_{ef} [cm ² /m]	i_{ef} [cm]			
0,75	0,107	377	377	12,5	5,49	8,91	5,26	6,29	8,46	7,75	9,69
0,88	0,126	446	446	14,8	5,49	8,91	7,13	6,24	8,48	10,0	12,5
1,00	0,143	510	510	16,9	5,49	8,91	9,04	6,21	8,51	11,4	14,3
1,25	0,179	642	642	21,3	5,49	8,91	13,6	6,08	8,68	14,4	18,0
1,50	0,215	775	775	25,7	5,49	8,91	18,4	5,96	8,82	17,4	21,7

values of shear field

t_N [mm]	$\min L_S$ [m]	z_{ulT_1} [kN/m]	z_{ulT_2} [kN/m]	$z_{ulT_3} = G_s / 750 [\text{kN}/\text{m}]$		K_3 ⁶⁾ [-]	z_{ulF_t} ⁷⁾		
				L_G ⁵⁾ [m]	$G_s = 10^4 / (K_1 + K_2 / L_S)$ [m/kN]		standartlength a		
							K_1 [m/kN]	K_2 [m ² /kN]	
0,75	4,7	1,48	2,06	6,3	0,308	54,9	0,67	9,00	12,0
0,88	4,4	1,90	3,14	5,3	0,260	36,0	0,73	10,6	14,2
1,00	4,1	2,33	4,38	4,7	0,228	25,8	0,78	12,2	16,2
1,25	3,6	3,29	7,81	3,7	0,181	14,5	0,87	15,3	20,5
1,50	3,3	4,36	12,5	3,3	0,150	9,05	0,96	18,5	24,7

Execution according DIN 18807 part 3, figure 6

0,75	4,7	1,48	2,06	6,3	0,308	54,9	0,67	9,00	12,0
0,88	4,4	1,90	3,14	5,3	0,260	36,0	0,73	10,6	14,2
1,00	4,1	2,33	4,38	4,7	0,228	25,8	0,78	12,2	16,2
1,25	3,6	3,29	7,81	3,7	0,181	14,5	0,87	15,3	20,5
1,50	3,3	4,36	12,5	3,3	0,150	9,05	0,96	18,5	24,7

Execution according DIN 18807 part 3, figure 7

0,75	5,0	3,71	1,99	5,2	0,308	33,0	1,04	9,00	12,0
0,88	4,6	4,78	3,03	5,2	0,260	21,7	1,04	10,6	14,2
1,00	4,3	5,84	4,23	5,3	0,228	15,5	1,04	12,2	16,2
1,25	3,8	8,26	7,54	5,5	0,181	8,70	1,04	15,3	20,5
1,50	3,5	11,0	12,1	5,1	0,150	5,44	1,04	18,5	24,7

¹⁾ Effective moment of inertia for direction of load downwards (+) or upwards (-).

²⁾ Coefficient width for constant comprehensive strength. $\sigma = f_{s,k}$

³⁾ Maximum spans the trapezoidal corrugated profile can be used as a loading component for roof systems.

These standart does not apply, whenever load distributioning arrangements (e.g. timber planks) are used to walk on profile.

⁴⁾ If shear field lengths $L_S < \min L_S$ the shear flow have to be reduced.

⁵⁾ If shear field lengths $L_S > L_G$ then z_{ulT_3} is not relevant.

⁶⁾ Support-contact forces $R_S = K_3 \cdot \gamma \cdot T$; (T = existing shear flow in [kN/m])

⁷⁾ Single load according to DIN 18807 part 3, chapter 3.6.1.5

Trapezoidal corrugated profile type:

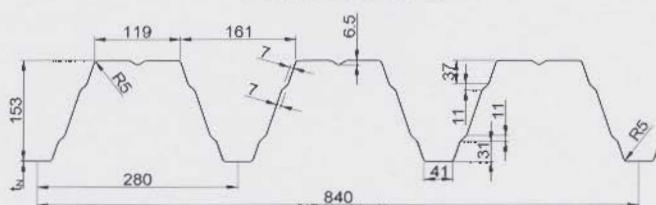
TR 150

Values of cross-section and resistance according DIN 18807 and
the "Anpassungsrichtlinie Stahlbau"

Position-positive

Measurements

in [mm]



Characteristic values of resistance
at down-pointing and pushing UDL¹⁾

thickness of plate t_N [mm]	panel moment $M_{F,k}$ [kNm/m]	Forces of end-support		intermediate support: method of proof E-E				intermediate support: method of proof P-P			
		load capacity $R_{A,T,k}$ [kN/m]	usability $R_{A,G,k}$ [kN/m]	interaction bending moment/support force ⁵⁾				residual support moment ⁶⁾			
				$\frac{\gamma_F \cdot M_{B,S,k}}{M_{B,k}^0 / \gamma_M} + \left(\frac{\gamma_F \cdot R_{B,S,k}}{R_{B,k}^0 / \gamma_M} \right)^\varepsilon \leq 1$	intermediate support force	$M_{R,k} = 0$ für $L \leq \min \ell$	$M_{R,k} = \frac{L - \min \ell}{\max \ell - \min \ell} \cdot \max M_{R,k}$	$M_{R,k} = \max M_{R,k}$ für $L \geq \max \ell$			
		$R_{A,T,k}$ [kN/m]	$R_{A,G,k}$ [kN/m]	$M_{B,k}^0$ [kNm/m]	$R_{B,k}^0$ [kN/m]	maxM _{B,k} [kNm/m]	maxR _{B,k} [kNm/m]	min ℓ [m]	max ℓ [m]	maxM _{R,k} [kNm/m]	
		²⁾ $b_A + \bar{U} = 40$ mm									
0,75	12,1	8,62	6,59	14,4	19,69	12,5	15,9	5,65	9,85	2,38	
0,88	18,0	13,7	10,5	24,3	26,87	16,4	21,7	5,87	11,3	3,58	
1,00	23,0	18,5	14,2	30,7	35,24	21,1	28,7	5,93	10,4	4,42	
1,25	29,4	27,2	20,8	33,9	58,22	33,9	47,4	5,84	8,06	5,35	
1,50	35,5	32,8	25,1	41,0	70,43	41,0	57,2	5,82	8,04	6,46	
		²⁾ $b_A + \bar{U} \geq 90$ mm									
0,75	12,1	11,2	8,57	14,1	36,65	12,5	29,7	3,67	9,55	3,81	
0,88	18,0	17,8	13,6	20,1	54,25	16,4	44,1	4,03	10,7	4,67	
1,00	23,0	24,1	18,4	25,4	71,57	21,1	57,9	4,24	9,94	5,51	
1,25	29,4	35,4	27,0	33,5	105,3	33,5	85,7	4,30	7,60	7,36	
1,50	35,5	42,7	32,6	40,4	127,8	40,4	103	4,27	7,56	8,88	

Characteristic values of resistance
at up-pointing and lifting UDL¹⁾

thickness of plate t_N [mm]	panel moment $M_{F,k}$ [kNm/m]	connection at every touching point				connection at every second belt					
		end-support $R_{A,k}$ [kN/m]	intermediate support, $\varepsilon =$			end-support $R_{A,k}$ [kN/m]	intermediate support, $\varepsilon = 1$				
			$M_{B,k}^0$ [kNm/m]	$R_{B,k}^0$ [kNm/m]	maxM _{B,k} [kNm/m]	maxR _{B,k} [kNm/m]	$M_{B,k}^0$ [kNm/m]	$R_{B,k}^0$ [kNm/m]	maxM _{B,k} [kNm/m]		
0,75	11,5	8,62	18,2	33,49	14,9	18,4	4,30	9,10	16,71	7,45	9,18
0,88	15,0	13,7	24,0	52,91	22,3	26,2	6,85	12,0	26,57	11,1	13,1
1,00	19,5	18,5	29,1	71,75	28,4	34,5	9,23	14,6	35,76	14,2	17,2
1,25	33,5	27,2	36,7	104,2	36,2	53,4	13,6	18,4	52,33	18,2	26,7
1,50	40,3	32,8	44,4	125,9	43,9	64,4	16,4	22,1	63,00	21,9	32,1

¹⁾ Instead of line load cross the direction of spanning and of single load the certificate has not to be determined by the field moment $M_{F,k}$, but with the support moment $M_{B,k}$ for the opponent direction of load

²⁾ $b_A + \bar{U}$ = end support width including profile overhang.

³⁾ For smaller support widths the characteristic values of resistance have to be reduced proportional.

If support widths is < 10mm, e.g. by pipes, 10 mm can be inserted.

⁴⁾ If support width is between the listed values it is allowed to interpolate the characteristic values of resistance linear.

⁵⁾ Interaction like pictured above. Are there no values for $M_{B,k}^0$ and $R_{B,k}^0$, $\gamma_F \cdot M_{B,S,k} \leq \max M_{B,k} / \gamma_M$ und $\gamma_F \cdot R_{B,S,k} \leq \max R_{B,k} / \gamma_M$ is essential.

⁶⁾ L = smaller of adjacent pitches. Are there no values for remaining pitch-moments, for structural safety certificate $M_{R,k} = 0$ or certificate according to elastic-elastic operation.

Trapezoidal corrugated profile type:

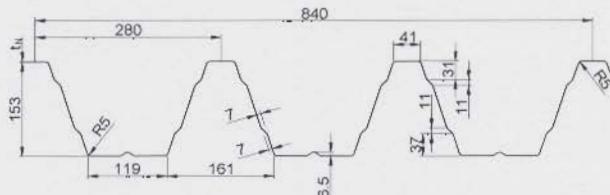
TR 150

Values of crosssection and resistance according DIN 18807 and
the "Anpassungsrichtlinie Stahlbau"

Position-negative

Measurements

in [mm]

Yield strength of steel core f_y, k 0 320 N/mm²

Values of cross-section

thickness of plate t_N [mm]	dead load g [kN/m ²]	bending ¹⁾		normal force stress					limit of span widths ³⁾		
		I_{ef}^+ [cm ⁴ /m]	I_{ef}^- [cm ⁴ /m]	not reduced cross-section			contributive cross-section ²⁾				
				A_g [cm ² /m]	i_g [cm]	z_g [cm]	A_{ef} [cm ² /m]	i_{ef} [cm]	z_{ef} [cm]	single field beam	multiple field beam
0,75	0,107	377	377	12,5	5,49	6,39	5,26	6,29	6,84	6,25	7,81
0,88	0,126	446	446	14,8	5,49	6,39	7,13	6,24	6,82	9,05	11,3
1,00	0,143	510	510	16,9	5,49	6,39	9,04	6,21	6,79	10,3	12,9
1,25	0,179	642	642	21,3	5,49	6,39	13,6	6,08	6,62	13,0	16,3
1,50	0,215	775	775	25,7	5,49	6,39	18,4	5,96	6,48	15,7	19,7

values of shear field

t_N [mm]	minL _S [m]	zulT ₁ [kN/m]	zulT ₂ [kN/m]	zulT ₃ =G _s /750[kN/m]		L _G [m]	K ₁ [m/kN]	K ₂ [m ² /kN]	zul F _t ⁷⁾				
				G _s =10 ⁴ /(K ₁ +K ₂ /L _S)					standartlength a				
				K ₃ ⁶⁾ [-]	≥ 130 mm [kN]				≥ 280 mm [kN]				
0,75	5,1	1,97	1,64	11,3	0,308	88,1	0,35		14,0	14,0			
0,88	4,7	2,54	2,50	11,4	0,260	57,9	0,38		16,6	16,6			
1,00	4,4	3,10	3,49	10,2	0,288	41,4	0,41		18,9	18,9			
1,25	3,9	4,39	6,22	8,1	0,181	23,2	0,46		23,9	23,9			
1,50	3,6	5,82	9,95	6,8	0,150	14,5	0,50		28,8	28,8			

Execution according DIN 18807 part 3, figure 6

0,75	5,1	1,97	1,64	11,3	0,308	88,1	0,35		14,0	14,0
0,88	4,7	2,54	2,50	11,4	0,260	57,9	0,38		16,6	16,6
1,00	4,4	3,10	3,49	10,2	0,288	41,4	0,41		18,9	18,9
1,25	3,9	4,39	6,22	8,1	0,181	23,2	0,46		23,9	23,9
1,50	3,6	5,82	9,95	6,8	0,150	14,5	0,50		28,8	28,8

Execution according DIN 18807 part 3, figure 7

0,75	2,1	9,94	11,3	2,1	0,308	2,00	1,26		14,0	14,0
0,88	2,0	12,8	17,2	2,0	0,260	1,32	1,26		16,6	16,6
1,00	1,8	15,6	24,0	1,8	0,228	0,942	1,26		18,9	18,9
1,25	1,6	22,1	42,9	1,6	0,181	0,528	1,26		23,9	23,9
1,50	1,5	29,3	68,6	1,5	0,150	0,330	1,26		28,8	28,8

¹⁾ Effective moment of inertia for direction of load downwards (+) or upwards (-).²⁾ Coefficient width for constant comprehensive strength. $\sigma = f_{y,k}$ ³⁾ Maximum spans the trapezoidal corrugated profile can be used as a loading component for roof systems.

These standart does not apply, whenever load distributioning arrangements (e.g. timber planks) are used to walk on profile.

⁴⁾ If shear field lengths $L_S < \text{minL}_S$ the shear flow have to be reduced.⁵⁾ If shear field lengths $L_S > L_G$ then zulT₃ is not relevant.⁶⁾ Support-contact forces $R_S = K_3 \cdot \gamma \cdot T$; (T = existing shear flow in [kN/m])⁷⁾ Single load according to DIN 18807 part 3, chapter 3.6.1.5

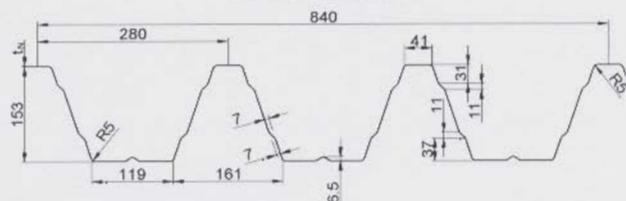
Trapezoidal corrugated profile type:

TR 150

Values of crossection and resistance according DIN 18807 and
the "Anpassungsrichtlinie Stahlbau"

Position-negative

Measurements
in [mm]



Characteristic values of resistance
at down-pointing and pushing UDL¹⁾

thickness of plate t _N [mm]	panel moment M _{F,k} [kNm/m]	Forces of end- support		intermediate support: method of proof E-E interaction bending moment/support force ⁵⁾				intermediate support: method of proof P-P residual support moment ⁶⁾			
		load capacity R _{A,T,k} [kN/m]	usability R _{A,G,k} [kN/m]	$\frac{\gamma_F \cdot M_{B,S,k}}{M_{B,k}^0 / \gamma_M} + \left(\frac{\gamma_F \cdot R_{B,S,k}}{R_{B,k}^0 / \gamma_M} \right)^{\varepsilon} \leq 1$	intermediat e support force						
						M _{B,k} [kNm/m]	R _{B,k} [kN/m]	maxM _{B,k} [kNm/m]	maxR _{B,k} [kN/m]	min ℓ [m]	max ℓ [m]
		²⁾ b _A +ü = 40 mm								³⁾ intermediate support width b _B = 60 mm, ε = 2	
0,75	11,5	8,62	6,59	12,9	20,01	12,9	16,2	5,12	8,52		2,94
0,88	15,0	13,7	10,5	19,6	28,42	19,6	23,1	4,61	7,26		3,90
1,00	19,5	18,5	14,2	24,7	37,42	24,7	30,5	4,73	7,19		4,67
1,25	33,5	27,2	20,8	30,5	57,99	30,5	47,1	5,68	8,17		5,93
1,50	40,3	32,8	25,1	36,7	70,27	36,7	56,9	5,65	8,14		7,17
		²⁾ b _A +ü ≥ 90 mm								⁴⁾ intermediate support width b _B ≥ 160 mm, ε = 2	
0,75	11,5	11,2	8,57	16,1	29,57	13,1	23,9	3,67	6,27		3,80
0,88	15,0	17,8	13,6	21,1	46,85	19,6	38,0	3,23	5,81		5,32
1,00	19,5	24,1	18,4	25,7	63,37	25,1	51,2	3,28	5,83		6,38
1,25	33,5	35,4	27,0	32,4	92,21	32,0	74,8	4,13	6,58		7,53
1,50	40,3	42,7	32,6	39,2	111,4	38,7	90,3	4,14	6,60		9,10

Characteristic values of resistance
at up-pointing and lifting UDL¹⁾

thickness of plate t _N [mm]	panel moment M _{F,k} [kNm/m]	connection at every touching point				connection at every second belt					
		end- support R _{A,k} [kN/m]	⁵⁾ intermediate support, ε =				end- support R _{A,k} [kN/m]	⁵⁾ intermediate support, ε = 1			
			M _{B,k} [kNm/m]	R _{B,k} [kN/m]	maxM _{B,k} [kNm/m]	maxR _{B,k} [kN/m]		M _{B,k} [kNm/m]	R _{B,k} [kN/m]	maxM _{B,k} [kNm/m]	maxR _{B,k} [kN/m]
0,75	12,1	8,62	15,9	41,47	14,2	18,0	4,30	7,97	20,75	7,09	9,04
0,88	18,0	13,7	22,8	61,60	18,5	24,6	6,85	11,4	30,83	9,25	12,3
1,00	23,0	18,5	28,7	80,89	24,0	32,5	9,23	14,4	40,60	12,0	16,2
1,25	29,4	27,2	37,9	119,4	37,9	53,7	13,6	19,0	60,15	19,0	26,9
1,50	35,5	32,8	45,7	144,7	45,7	64,8	16,4	23,0	72,42	23,0	32,5

¹⁾ Instead of line load cross the direction of spanning and of single load the certificate has not to be determined by the field moment M_{F,k}, but with the support moment M_{B,k} for the opponent direction of load

²⁾ b_A + ü = end support width including profile overhang.

³⁾ For smaller support widths the characteristic values of resistance have to be reduced proportional.

If support widths is < 10mm , e.g. by pipes, 10 mm can be inserted.

⁴⁾ If support width is between the listed values it is allowed to interpolate the characteristic values of resistance linear.

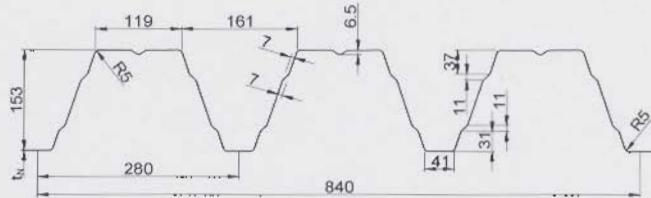
⁵⁾ Interaction like pictured above. Are there no values for M_{B,k}⁰ and R_{B,k}⁰, γ_F · M_{B,S,k} ≤ maxM_{B,k} / γ_M und γ_F · R_{B,S,k} ≤ maxR_{B,k} / γ_M is essential.

⁶⁾ L = smaller of adjacent pitches. Are there no values for remaining pitch-moments, for structural safety certificate M_{R,k} = 0 or certificate according to elastic-elastic operation.

Stress-table for an evenly distributed load

Table of profile in positive position

Measurements
in [mm]



The trapezoidal corrugated profile as component is for individual persons only walkable across load distribution arrangements (e.g. timber planks).

An accordant advice had to be made on laying plan.

Line *: Maximum permissible stress without limitations by deflection.

Line L/....: Permissible stress under additional limitation by deflection – max f ≤ L/.... .

These values are also valid for the lower part of the table for two- and three-field beams if they are lower than those in line *.

The tables do not replace the necessary statically building construction certificate. The details for permissible stress in [kN/m²] had been determined according to regulations of DIN 18807 and the adaptation guideline for structural steelwork.

Particularly at multiple field beams the maximum supply length should be considered.

Single field beams, permissible uniformly distributed load (UDL) [kN/m²]

		widths of end-supports: b _A = 90 mm																								
t _f	g	max f		3,50	3,75	4,00	4,25	4,50	4,75	5,00	5,25	5,50	5,75	6,00	6,25	6,50	6,75	7,00	7,25	7,50	7,75	8,00	8,25	8,50	8,75	9,00
0,75	10,7	*		3,87	3,61	3,39	3,19	2,90	2,60	2,35	2,13	1,94	1,77	1,63	1,50	1,39	1,29	1,20	1,12	1,04	0,98	0,92	0,86	0,81	0,77	0,72
		L/150		3,87	3,61	3,39	3,19	2,90	2,60	2,35	2,13	1,94	1,77	1,63	1,50	1,39	1,29	1,18	1,06	0,96	0,87	0,79	0,72	0,66	0,61	0,56
		L/300		3,87	3,61	3,16	2,64	2,23	1,89	1,62	1,40	1,22	1,07	0,94	0,83	0,74	0,66	0,59	0,53	0,48	0,44	0,40	0,36	0,33	0,30	0,28
		L/500		2,84	2,31	1,90	1,58	1,34	1,14	0,97	0,84	0,73	0,64	0,56	0,50	0,44	0,40	0,35	0,32	0,29	0,26	0,24	0,22	0,20	0,18	0,17
0,88	12,6	*		6,14	5,73	5,38	4,83	4,31	3,87	3,49	3,17	2,89	2,64	2,42	2,23	2,07	1,92	1,78	1,66	1,55	1,45	1,36	1,28	1,21	1,14	1,08
		L/150		6,14	5,73	5,38	4,83	4,31	3,87	3,49	3,17	2,88	2,52	2,22	1,96	1,75	1,56	1,40	1,26	1,14	1,03	0,94	0,85	0,78	0,72	0,66
		L/300		5,59	4,55	3,74	3,12	2,63	2,24	1,92	1,66	1,44	1,26	1,11	0,98	0,87	0,78	0,70	0,63	0,57	0,52	0,47	0,43	0,39	0,36	0,33
		L/500		3,36	2,73	2,25	1,87	1,58	1,34	1,15	0,99	0,86	0,76	0,67	0,59	0,52	0,47	0,42	0,38	0,34	0,31	0,28	0,26	0,23	0,21	0,20
1,00	14,3	*		8,31	7,76	6,97	6,17	5,51	4,94	4,46	4,05	3,69	3,37	3,10	2,85	2,64	2,45	2,28	2,12	1,98	1,86	1,74	1,64	1,54	1,46	1,38
		L/150		8,31	7,76	6,97	6,17	5,51	4,94	4,39	3,79	3,30	2,88	2,54	2,25	2,00	1,78	1,60	1,44	1,30	1,18	1,07	0,98	0,89	0,82	0,75
		L/300		6,40	5,21	4,28	3,57	3,01	2,56	2,19	1,89	1,65	1,44	1,27	1,12	1,00	0,89	0,80	0,72	0,65	0,59	0,54	0,49	0,45	0,41	0,38
		L/500		3,84	3,12	2,57	2,14	1,81	1,54	1,32	1,14	0,99	0,86	0,76	0,76	0,67	0,60	0,53	0,48	0,43	0,39	0,35	0,32	0,29	0,27	0,25
1,25	17,9	*		11,64	10,14	8,91	7,89	7,04	6,32	5,70	5,17	4,71	4,31	3,96	3,65	3,37	3,13	2,91	2,71	2,53	2,37	2,23	2,09	1,97	1,86	1,76
		L/150		11,64	10,14	8,91	7,89	7,04	6,32	5,52	4,77	4,15	3,63	3,20	2,83	2,51	2,24	2,01	1,81	1,64	1,48	1,35	1,23	1,12	1,03	0,95
		L/300		8,05	6,55	5,39	4,50	3,79	3,22	2,76	2,38	2,07	1,81	1,60	1,41	1,26	1,12	1,01	0,91	0,82	0,74	0,67	0,61	0,56	0,52	0,47
		L/500		4,83	3,93	3,23	2,70	2,27	1,93	1,66	1,43	1,24	1,09	0,96	0,85	0,75	0,67	0,60	0,54	0,49	0,44	0,40	0,37	0,34	0,31	0,28
1,50	21,5	*		14,05	12,24	10,76	9,53	8,50	7,63	6,88	6,25	5,69	5,21	4,78	4,41	4,07	3,78	3,51	3,27	3,06	2,87	2,69	2,53	2,38	2,25	2,12
		L/150		14,05	12,24	10,76	9,53	8,50	7,63	6,67	5,76	5,01	4,38	3,86	3,41	3,03	2,71	2,43	2,19	1,98	1,79	1,63	1,48	1,36	1,24	1,14
		L/300		9,72	7,91	6,51	5,43	4,57	3,89	3,33	2,88	2,50	2,19	1,93	1,71	1,52	1,35	1,21	1,09	0,99	0,90	0,81	0,74	0,68	0,62	0,57
		L/500		5,83	4,75	3,90	3,26	2,74	2,33	2,00	1,73	1,50	1,31	1,16	1,02	0,91	0,81	0,73	0,66	0,59	0,54	0,49	0,45	0,41	0,37	0,34

